

# Critical Buckling Load Analysis for Isotropic Laminated Composite Plates

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**Abstract:** The purpose of this examination is to evaluate the conforming heap of isotropic that is subjected to pressure in the plane. ANSYS 19.0 was the investigation tool that was employed for the purpose of this inquiry. Altering the parameters, such as the angle proportion ( $a/b$ ), the thickness proportion ( $S$ ), and the limit conditions, allows for the evaluation of the clasping load imposed on the material. It was observed that the basic clasping load was affected by the different proportions of length to width, and that the fiber direction edges also had an effect on the basic clasping load symmetric point use plates.

**Keywords:** Buckling Analysis, Laminated Plates, Fibre Reinforced Polymers.

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## I. INTRODUCTION

These days, composite overlay constructions are often used due to their advantageous qualities, such as their insurance against consumption and a tolerably high quality to weight extent. The use of overlay composite constructions is becoming more and more popular as a result. Their sensitive nature in the through-the-thickness bearing is a significant problem, mostly due to their intrinsic low grip interlinearity. Their dissatisfaction results from both excessive concerns and buckling in many construction structures, such as segments, shafts, or plates. In the present research, only square, fragile plates are taken into consideration. A level plate remains level and in a balanced state when subjected to modest in-plane compressive loads. In any case, the plate's harmonic arrangement always changes to a non-level design as the in-plane pressure load increases, making the plate ultimately unstable. The term "basic buckling load" refers to the compressive burden size at which the plate becomes unstable.

Given its high caliber to weight and high solidity to weight degrees, a composite material, which consists of at least two components, provides a fundamental weight reduction in buildings.

Furthermore, by appropriately controlling the strands' orientation, the mechanical characteristics of a fiber composite may be altered as needed. The lattice, which has a low modulus and a high expansion, provides the fundamental adaptability in such materials, while also holding the filaments in place and protecting them from the ground. The fibers in such materials are the typical burden-bearing individuals.

Composite materials are unavoidably made up of two elements that have solidified to form a material having unexpected qualities compared to the constituents' separate properties. Two components make up fiber-fortified plastic (FRP), a composite material: a series of strands encased in a framework. A stacking lamina delineates a FRP cover, whereas a layer of composite material is referred to as a lamina. FRP has been used for a long time in the automotive and aviation industries, and it has recently been employed as an alternative to steel, wood, and cement in structural construction.

The foundational theoretical analysis of versatile flexural-torsional buckling was previously provided by Holy Person Venart's 1855 journal on uniform torsion, which provided the crucially important strong illustration of the turning response of humans to torsion, and Euler's (1759) treatise on area flexural buckling, which provided the basic explanatory strategy for anticipating the diminished characteristics of slim sections. Chen and Bert (1976) examined the optimal arrangement of rectangular plates that were essentially reinforced, covered with composite material, and subjected to uniaxial compressive stacking. For ideal structure plates with glass/epoxy, boron/epoxy, and carbon/epoxy composite materials superimposed, numerical results are shown. Yeh and Tooth (1997) analyzed covered plates with strip-type delamination under twisting in a rational and likely manner. Radu and Chattopadhyay (2000) dismantled the dynamic precariousness associated with composite plates with delamination that are susceptible to dynamic compressive loads using a revised greater request shear twisting concept. In order to predict the delamination buckling burden and delamination development load, Hwang and

Mao (2001) guided the non-straight buckling and post-buckling evaluations. Wen-pie and Lin Cheng (2003) provide a potent inspection display to get a buckling heap of plate. Wang and Lu (2003) completed an evaluation to look at the buckling behavior of localized delamination along the exterior of fiber-strengthened overlay plates under thermal and mechanical stresses.

In order to measure the basic/buckling heaps of covered composite rectangular plates under in-plane uniaxial and biaxial loadings, Shukla and Kreuzer (2005) presented a method that relies on the primary request shear twisting speculation and von-Karman-type nonlinearity. Using the Ritz system near the suggested out-of-plane boundary conditions, Pankok and Singhhatanadgid (2006) study the buckling conduct of rectangular and slant dainty composite plates with different boundary circumstances.

Buket Okutan Baba (2007) examined how boundary circumstances affected the rectangular plates' buckling load. Pein and Zahari (2007) examined the fundamental behavior of woven texture composites under inadequate compressive load. To illustrate non-straight material conduct and capture the overall compressive reaction of woven composite plates made of glass-epoxy material, Zahari and Azmee (2008) developed a dynamic disappointment examination figure that is implemented as a client subroutine in a limited component code (ABAQUS). The goal of the present analysis is to analyze how different boundary conditions, perspective proportions, thicknesses, and utilize directions affect buckling and post-buckling behavior. Because these plates may have been fragmented for various boundary conditions, the buckling behavior of plates should be thoroughly investigated. In this sense, the inspection is essential with the specific goal of understanding how these plates buckle.

## II. PROBLEM STATEMENT

The basic buckling heap of several overlapping plates is broken down using ANSYS to see how variations in the covered plate might have affected the buckling load. The thickness, boundary condition, perspective percentage, and direction of the stitched tangle layers used in FRP were the four variables that determined the progressions to the covered plate. Under the following boundary condition, the covered plates were broken down: essentially simply essentially. Similar to how it was done for the isotropic plates and will be found in the demonstration area, the boundary condition was applied to the plate's edge hubs. Three distinct plate thicknesses—8 mm, 1 omm, and 12 mm—were used. There were four different angle proportions (a/b) observed: 1.25, 1.5, 2.o, and 1.

**Table 1** Longitudinal and transverse modulus of laminated plates for different orientation:

	15	30	45	60
Ex (Mpa)	23595	20685.28	18037.69	16934.52
Ey (Mpa)	10902	11087.77	12342.62	14969.52
Poison's ratio	0.25	0.25	0.25	0.25
Shear modulus, Gx (Mpa)	4140	4140	4140	4140
Shear modulus, Gy (Mpa)	3450	3450	3450	3450

Bending stiffness for (+15/-15/-15/+15) orientation

**Table 2** Bending Stiffness Matrix D Mpa

Plates thickness (mm)	D <sub>11</sub>	D <sub>22</sub>	D <sub>66</sub>	D <sub>12</sub>
8	1073796	496120.3559	147201.15	124031.1103
10	2097159	968938.7409	287489.5	242235.7269
12	3624034	1674394.12	496800	418599.352

Bending stiffness for (+30/-30/-30/+30) orientation

**Table 3** Bending Stiffness Matrix D Mpa

Plates thickness (mm)	D <sub>11</sub>	D <sub>22</sub>	D <sub>66</sub>	D <sub>12</sub>
8	941371.9	504575.822	147201.15	126143.7908
10	1838528	985451.5804	287488.5	246363.8118
12	3177105	1702927.72	496801	425731.968

### III. RESULTS

In the current inquiry, the aftereffects of a restricted component analysis of composite covered plates are shown. Through the use of the discrete model technique, the limited component model is applied. The restricted component software known as ANSYS was used in order to take into consideration the replicated conduct of the plates as well as the contrasting and hypothetical definition it provided. When it comes to the management of dedicated numerical models for the buckling conduct of composite covered plates under static conditions, the program ANSYS was designed to be suitable. For the purpose of displaying the composite overlay plate, eight-hub strong block components (Shell 281) were employed. SHELL281 is suitable for dissecting shell structures that range from somewhat thin to substantial in thickness. There are eight hubs in the component, and each hub offers six degrees of possibility. These degrees include interpretations in the x, y, and z tomahawks, as well as pivots around the x, y, and z-tomahawks individually.

There is only the possibility of translational degrees of opportunity when the component is used in conjunction with the film option. SHELL281 is suitable for applications that include nonlinearities with a large amount of strain, as well as those that involve straight revolutions. In nonlinear research, fluctuations in shell thickness are represented as a variable. The devotee (load firmness) effects of distributed pressures are represented by the component. It is possible to use SHELL281 for layered applications, either to demonstrate the construction of composite shells or to demonstrate sandwich development. When it comes to showing composite shells, the primary request shear-twisting theory, which is also often referred to as the Mindlin-Reissner shell hypothesis, is responsible for ensuring accuracy. The definition of the component is dependent on the logarithmic strain as well as the real pressure measurements. Taking into account restricted layer stresses (extending) is something that the component kinematics take into mind. In spite of this, the arch alterations that occur during a period improvement are considered to be quite minor.

Critical Buckling Load for Isotropic Plates (SS)

**Table -4**

Length (a) in mm	Breadth (b) in mm	Aspect ratio (a/b)	Thickness in mm	Calculated critical buckling load N/mm	ANSYS critical buckling load N/mm	Percentage difference
4000	1000	1	8	370.196	368.42	0.48%
4000	1000	1	10	723.047	718.63	0.61%
4000	1000	1	12	1249.41	1240.12	0.74%

Critical buckling load for laminated plate (15/-15/-15/15)

**Table-5**

A in mm	B in mm	Aspect ratio (a/b)	Plate thickness in mm	Calculated critical buckling load in Mpa	ANSYS critical buckling load in Mpa	Percentage error
1000	1000	1	8	23.74	25.20	6.14%
1000	800	1.25	8	34.512	38.25	10.83%
1000	666.667	1.5	8	53.94	58.27	10.1%
1000	500	2	8	94.512	100.51	6.21%
1000	1000	1	10	46.38	49.11	6.35%
1000	800	1.25	10	67.40	74.52	6.45%
1000	666.667	1.5	10	105.36	113.48	6.84%
1000	500	2	10	185.55	195.48	5.35%
1000	1000	1	12	80.143	84.66	5.62%
1000	800	1.25	12	116.45	128.41	10.31%
1000	666.667	1.5	12	182.07	195.53	7.14%
1000	500	2	12	320.64	336.28	4.9%

#### IV. CONCLUSION

Within the scope of this inquiry, the buckling response of overlaying rectangular plates with a variety of boundary circumstances is taken into consideration. Changing thickness, direction, and perspective proportions are characteristics of the composite plates that have been superimposed. Beginning with the current approach, it is possible to arrive to the accompanying endings. It was observed that the fundamental buckling stress was affected by the different proportions of length to expansiveness inside the material. The percentage of a to b increases, which results in an increase in the buckling load. The variation in buckling load is about 24 percent at the point in time when the viewpoint proportion changed from 0.5 to 1. When the angle proportion is taken into consideration, the rate of progression of the buckling load is about the same.

#### REFERENCES

- [1]. Singer J., Arbocz J., Wetter T., 'Buckling experiments: Experimental methods in buckling of thin walled structures: shells, Built up structures, composites and additional topics', vol.2. , New York: John Willey and sons; (2002).
- [2]. Reddy J.N., second edition, 'Mechanics of laminated composite plates and shells', Boca Raton CRC press; (2004).
- [3]. Whitney J.M., 'Structural analysis of laminated anisotropic plates', Lancaster, PA: Technomic Publishing; (1987).
- [4]. Reddy J.N., 'Mechanics of laminated composite plates: theory and analysis', Boca Raton: CRC press; (1997).
- [5]. Leissa A.W., Kang J., 'Exact solutions for vibration and buckling of an SS – C – SS – C rectangular plate loaded by linearly varying in – plane stresses', International Journal of mechanical sciences;(2002),44:PP.1925.
- [6]. Bao G., Jiang W., Roberts J.C., 'Analytic and finite element solutions for bending and buckling of orthotropic rectangular plates', International Journal of solids and structures ; (1997),34(14),PP.1792.
- [7]. Robinson J.R., 'The buckling and bending of orthotropic sandwich panels with all edges- simply supported', Aero Q; (1955), 6(2):PP. 125.
- [8]. Baharlou B., Leissa A.W., 'Vibration and buckling of generally laminated composite plates with arbitrary edge conditions', International Journal of mechanical sciences;(1987),29(8):PP.545.
- [9]. Dawe D.J., Wang S., 'Spline finite strip analysis of the buckling and vibration of rectangular composite laminated plates', International Journal of mechanical sciences; (1995),37(6):PP.645.
- [10]. Liu G.R., Chen X.L., Reddy J.N., 'Buckling of symmetrically laminated composite plates using the element free Galerkin method', International Journal of structural stability dynamics; (2002), 2(3):PP.281.
- [11]. Bert C.W., Malik M., 'Differential quadrature: A powerful new technique for analysis of composite structures', composite structures; (1997),39(3– 4):PP.179.
- [12]. Huang Y.Q., Li Q.S., 'Bending and buckling analysis of anti – symmetric laminates using the least square differential quadrature method', Computer methods in applied mechanics and engineering, 193; (2004):PP.3471.
- [13]. Kim Y.S., Hoa S.V., 'Biaxial buckling behaviour of composite rectangular plates', composite structures; (1995), 31(4):PP.247.
- [14]. Shufrin I., Rabinovitch O., Eisenberger M., 'Buckling of symmetrically laminated rectangular plates with general boundary conditions – A semi analytical approach', composite structures; .
- [15]. Kerr A.D., 'An extended Kantorovich method for solution of Eigen value problem', International Journal of solids and structures; (1969),5 (7):PP.559.
- [16]. Eisenberger M., Alexandrov A., 'Buckling loads of variable thickness thin isotropic plates', thin – walled structures; (2003), 41(9):PP.871.
- [17]. Shufrin I., Eisenberger M., 'stability and vibration of shear deformable plates – first order and higher order analyses', International Journal of solid structures; (2005), 42(3 – 4):PP.1225.
- [18]. Ungbhakorn V., singhatanadgid P., 'Buckling analysis of symmetrically laminated composite plates by extended Kantorovich method', Composite structures; (2006); 73(1):PP.120.
- [19]. Yuan S., Jin Y., 'Computation of elastic buckling loads of rectangular thin plates using the extended Kantorovich method', computer structures; (1998), 66(6):PP.861 – .
- [20]. March H.W., Smith C.B., 'Buckling loads of flat sandwich panels in compression', Forest products research laboratory report No.1525, Madison, WI; (1945).
- [21]. Chang C.C., Ebcioğlu I.K., Haight C.H., 'General stability analysis of orthotropic sandwich panels for four different boundary conditions', Zeitschr Angew, Math. Mech.; (1962), 43:PP.373.
- [22]. Jiang W., Bao G., Roberts J.C., 'Finite element modeling of stiffened and unstiffened orthotropic plates', Computer and structures Journal; (1977), 63(1):PP.105.
- [23]. Smith C.S., 'Design of marine structures in composite materials', Amsterdam: Elsevier science publishers Ltd; (1990).